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# US 250/Wilson Workforce and Rehabilitation Center Small Area Study 

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## US 250/WWRC Small Area Study

## Disclaimer

This report has been prepared in cooperation with, and financed in part, by the U.S. Department of Transportation - Federal Highway Administration, the Federal Transit Administration, the Virginia Department of Transportation, and the Virginia Department of Rail and Public Transportation. The contents of this report reflect the views of the Central Shenandoah Planning District Commission and Staunton-Augusta-Waynesboro Metropolitan Planning Organization, which are responsible for the facts and accuracy of the data presented herein. The contents do not necessarily reflect the official views or policies of the Federal Highway Administration, Federal Transit Administration, the Virginia Department of Transportation, or the Virginia Department of Rail and Public Transportation. This report is not a legal document, and does not constitute a standard, specification, or regulation. Although much care was taken to ensure the accuracy of information presented in this document, CSPDC does not guarantee the accuracy of this information.

Acceptance of this report as evidence of fulfillment of the objectives of this planning study does not constitute endorsement/approval of the need for any recommended improvement, nor does it constitute approval of their location and design or a commitment to fund any such improvements. Additional project level environmental impact assessments and/or studies of alternatives may be necessary.

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## US 250/WWRC Small Area Study

## Executive Summary

## Purpose

The US 250/Wilson Workforce and Rehabilitation Center (WWRC) Small Area Study conducted by the Staunton-Augusta-Waynesboro Metropolitan Organization (SAWMPO) in collaboration with Augusta County, the Virginia Department of Transportation (VDOT) Staunton District, and the Timmons Group, analyzes operational and safety conditions at the entrance to the WWRC Campus to identify short- and longterm improvement strategies to reduce congestion and delay at the entrance to the WWRC Campus and to identify potential locations for a second entrance to the WWRC Campus.

## Study Team

The study team included staff from the SAWMPO, Augusta County, VDOT, and the Timmons Group.

## Analysis

The study examines the US 250 corridor between Idlewood Boulevard and Barren Ridge Road in Fishersville, VA to assess existing and forecast future corridor operating conditions. Data examined include traffic counts, turning movements, delays, crashes, and field observations. Using this data, a traffic model of the study area was developed to assess short- and long-term no-build and improvement scenarios.

## Improvement Recommendations

Short-term improvement recommendations focus on operational improvements at the entrance to the WWRC Campus. Long-term improvement options identify potential second entrance locations to the WWRC Complex. Short-term improvement recommendations are illustrated on Figures 10 and 11. Long-term improvement options are illustrated in Figure 12. Short- and long-term improvement scenarios with planning level cost estimates are provided in Section 5. Detailed cost estimates are found in Appendices B and C.


## US 250/WWRC Small Area Study

## Short-term Improvements

Short-term improvement recommendations were developed to enhance operational and safety conditions at the entrance to the WWRC Complex by alleviating congestion and reducing vehicle delays and queues during the AM and PM peak hours. Analysis of study area traffic data demonstrated that the following improvements address the study's stated goals and maintain the existing non-motorized facilities parallel to Woodrow Wilson Avenue.

1. Add a channelized westbound right-turn lane at the WWRC Complex entrance on US 250 and an associated northbound receiving lane on Woodrow Wilson Avenue
2. Add a separate southbound right-turn lane on Woodrow Wilson Avenue at the WWRC Complex US 250 entrance
3. Adjust signal timing at the US 250/Woodrow Wilson Avenue/LifeCore Drive intersection

While the short-term recommendations improve operating conditions at the 250 /Wilson/Lifecore intersection, a single entrance to the WWRC facility will not adequately serve the long-term study area traffic demands.

## Long-term Options

The long-term goal is to add a new entrance to the WWRC Complex. The study team identified 13 possible road alignments and narrowed the number of possible alignment options to three during the study process. The study team analyzed the three second entrance options to determine how each improved future corridor operating conditions, and to develop planning level cost estimates. Implementation of a long-term option requires the construction of a new roadway and associated improvements at the new connection on US 250. Long-term options are detailed in Section 4.2 and are shown on Figure 10.

## Public Outreach

The study team held two public outreach meetings at Wilson Middle School to provide the community with the opportunity to learn about the study, review study materials, and provide input. Public meeting notices were posted in the Staunton News Leader and Waynesboro News Virginian, on the SAWMPO web site, and were provided to the schools in the WWRC Complex for distribution to parents and staff. Press releases for each event were provided to local media outlets. Flyers were distributed to businesses located within the study area. Copies of the public notices and flyers are in Appendix A. To view the study webpage please go to www.sawmpo.org.

The public meetings were held on:


- June 22, 2017
- January 31, 2018


## 1. Background

### 1.1 Purpose and Need

The SAWMPO conducted a Small Area Transportation Study to examine conditions and identify improvement solutions at the entrance to the WWRC Complex at the intersection of US 250 and Woodrow Wilson Avenue (VA 358)/LifeCore Drive (VA 636). Currently, vehicles experience congestion and delays at the entrance to the WWRC Complex during peak AM and PM hours, and the Campus lacks a secondary/emergency access road. The study area is a one-mile segment of the US 250 corridor between Idlewood Boulevard and Barren Ridge Road in the
 community of Fishersville, VA.

The purpose of this study is to:

- Inventory and summarize existing conditions.
- Collect and analyze traffic data for existing and forecasted operational conditions.
- Provide short-term improvement recommendations and long-term improvement options.
- Provide planning level cost estimates for improvement recommendations presented in this study.

The study was led by the SAWMPO in partnership with the Virginia Department of Transportation (VDOT), Augusta County, and the Timmons Group.

### 1.2 Corridor Operational and Safety Issues

Operational and safety issues in the study area include congestion and delays caused by the WWRC complex schools and institutions' concentrated demand on the single point of access at the US 250-Woodrow Wilson Avenue intersection during the AM and PM peak hours.

Traffic data analysis, field observations, and input from stakeholders, which include representatives from the WWRC, Augusta County, Augusta County Schools, and public meeting participants, identified the following operational and safety issues in the study area.

Extensive vehicle queues often occur at the entrance to the WWRC Complex in the US 250 westbound right-turn lane. Queues for vehicles turning right to enter the WWRC Complex were observed extending approximately 1,800feet from the entrance during the AM peak hour.

1. Delays on Woodrow Wilson Avenue for southbound traffic exiting the WWRC Complex occur during AM and PM peak hours. Southbound through-traffic on Woodrow Wilson Avenue shares the lane with traffic turning right at the intersection. This shared through/right-turn lane contributes to the development of extensive queues during AM and PM peak hours. Additionally, left-turning traffic exiting the WWRC Complex was observed to queue past Wilson Elementary School (approximately 1,400-feet from the WWRC Complex entrance) and block the southbound through/right-turn lane at US 250.

2. There is no secondary or emergency access point to the Complex.

### 1.3 Study Assumptions

The following assumptions were made for this study:

1. Annual Growth Rate: The study team determined annual growth rates by reviewing historical traffic counts, the VDOT state planning system forecast and anticipated future development within the study area. The following growth rates were used for the traffic modeling analysis:

- US 250: 1.0\%
- VA 358: 0.5\%
- VA 636: 2.0\%

2. The horizon year for short-term improvements is 2022. The horizon year for a long-term option is 2040. Analysis for long-term options include having short-term improvements in place at time of implementation.

### 2.0 Study Area Characteristics

Roads in the study area include US 250, Woodrow Wilson Avenue (VA 358), LifeCore Drive (VA 636) and Barren Ridge Road.

US 250 is a five-lane undivided minor arterial highway. The speed limit is $45-\mathrm{mph}$ through the study area. US 250 is identified for potential inclusion in the VDOT Arterial Preservation Network, pending CTB approval.

Woodrow Wilson Avenue (VA 358) is a two-lane, two-way road and provides the only entrance to the WWRC Complex. The posted speed is $40-\mathrm{mph}$ at the entrance and decreases to $25-\mathrm{mph}$ at Wilson Elementary School, approximately 1,200 -feet north from the entrance.


LifeCore Drive (VA 636) is two-lane, two-way road on the south side of the intersection at the WWRC Complex entrance. LifeCore Drive is classified as a major collector road with a posted speed limit of $45-\mathrm{mph}$, and provides a link to I-64 to the south. The intersection of US 250 with Woodrow Wilson Avenue (VA 358) and Lifecore Drive (VA 636) is signal-controlled with dedicated left turn lanes and flashing yellow arrows for each direction on US 250. US 250 eastbound has two through and one free-flow right turn lane. US 250 westbound has two through lanes, in addition to the left turn lane. Woodrow Wilson Avenue has one through- and two left-turn lanes at the intersection, but no dedicated right turn lane. Lifecore Drive has two left-turn lanes, and a through/right turn lane at the intersection.

Barren Ridge Road is a two-lane, two-way road located approximately 3,455-feet ( 0.65 -miles) to the east of the WWRC Complex entrance. Barren Ridge Road connects US 250 to surrounding agricultural and residential uses. The intersection of US 250 with Barren Ridge and Mule Academy Roads is signal-controlled with dedicated through- and left-turn lanes and shared through/right turn lanes in both directions on US 250. Barren Ridge Road has a single lane serving all movements at the intersection. Mule Academy provides a dedicated right-turn lane, and a through/left turn lane.

Development in the study area includes the WWRC Complex, several retail centers, office and medical buildings, single- and multi-family development, recreational facilities and agricultural uses. Facilities at the WWRC Complex include the Wilson Workforce and Rehabilitation Center, three public schools (Wilson Elementary, Wilson Middle, and Wilson High School), the Governor's School, the Valley Career Technical Center, the Augusta County Schools vehicle maintenance center, recreational facilities, and agricultural and residential uses. The entrance to the WWRC Complex is located at the intersection of US 250 and Woodrow Wilson Avenue (VA 358). US 250 also serves as an alternate/detour route for I-64. The study area is shown in Figure 1.

## US 250/WWRC Small Area Study

Study area non-motorized facilities include The Coleman Career Pathway, a 10-foot wide shared use recreational path that runs parallel along the western side of Woodrow Wilson Avenue/LifeCore Drive. This path crosses US 250 at a signalized crosswalk at the entrance to the WWRC Complex. The shared use path connects the WWRC complex and its recreational facilities to the LifeCore Drive Shared Use Path.

Transit service in the study area is available Monday - Saturday via BRITE Transit's 250 Connector Route. Bus stops are located at the corner of Idlewood Boulevard and US 250, and in the WWRC Complex at the WWRC main offices.

## US 250/WWRC Small Area Study

Figure 1: WWRC Study Area


### 2.1 Zoning

Zoning designations in the study area include General Agriculture (GA), General Business (GB), Multi-Family (MF), Single Family (SF), and Mobile Home Park (MHP). Figure 2 illustrates zoning designations in the study area.

Figure 2: Zoning in Study Area


### 2.2 Environmental

This section does not constitute an environmental review or other such level of site assessment. It has not been determined whether a future a project will require an environmental assessment or a categorical exclusion to meet the requirements of the National Environmental Policy Act (NEPA).

### 2.2.1 Soils

The study area is underlain by a variety of soil types with the Chilhowie-Edom Complex and Weikert-Berks Channery soil types being dominate in the study area. Figure 3 maps the soil types and locations within the study area.

### 2.2.2 Geology

The study area is in the Ridge and Valley Ecoregion, and the 67 a - Northern Limestone/Dolomite Valleys sub-ecoregion. The geologic subsurface in the study area is shale. Shale tends to heave when moisture increases, or during freeze-thaw conditions. Heaving loosens material and may impact soil stability causing them to lose compaction and density. Figure 4 maps the geology of the study area.

### 2.3 Wetlands

There are several small wetland areas and water bodies in the study area. A jurisdictional determination will be required to quantify all streams and wetland areas in the study area.

### 2.4 Historic Resources

A Department of Historic Resources (DHR) project review was not requested at this preliminary study stage. A DHR project review may be required if a project is pursued in the study area. It is important to note that the WWRC, which includes early twentieth century structures, is in the study area.

Figure 5 identifies jurisdictional waters, wetlands, and historic resources in the study area.

### 2.5 Utilities

### 2.5.1 Water/Sewer Service

Study area water and sewer service is provided by the Augusta County Service Authority (ACSA) South River Water System. Water and sewer trunk lines run parallel to US 250 and provide service connections in the study area and are identified in Figure 6.

### 2.5.2 Electrical Service

Dominion Power owns and operates the Electric utilities. Power lines are mounted on standard timber stock poles. Power lines are located overhead and underground within the WWRC Complex.

Figure 3: Soils


## US 250/WWRC Small Area Study

Figure 4: Geology


## US 250/WWRC Small Area Study

Figure 5: Historic Structures, Hydrology \& Wetlands


US 250/WWRC Small Area Study

Figure 6: Water \& Sewer Service


## 3. Operating Conditions

Transportation facilities examined for this study are US 250, and the US 250 intersections at Woodrow Wilson Avenue/LifeCore Drive, and Barren Ridge Road. Operating conditions examined include existing roadway characteristics, traffic counts, turning movements, and crashes.

### 3.1 Roadway Characteristics

### 3.1.1 U.S. 250

The segment of US 250 in the study area is a minor arterial east-west roadway situated between the City of Waynesboro to the east and the City of Staunton and I-81 to the west. The posted speed limit in the study area is 45 miles-per-hour (mph). This section of US 250 serves both the local and regional transportation network and serves as an alternate/detour route for I-64. Topographical features in the study area include vertical curvatures that may present sight distance issues for drivers.


This segment could potentially be included in the State's Arterial Preservation Network (APN). Arterial Preservation Network routes are designated to accommodate long-distance mobility for people and goods throughout the Commonwealth. MES designated corridors are roadway segments inside urban areas (population of 50,000 or more) where opportunities exist to reduce congestion and improve traffic flow without major roadway widening. This designation influences corridor access management by requiring additional review and justification measures to install new driveway/entrance locations and traffic signals. The APN designation for the US 250 Corridor is pending adoption by the Commonwealth Transportation Board.
Multiple entrances and side roads are located throughout the US 250 corridor study area, with most of the commercial entrances located on the southern side of US 250 between the WWRC Complex entrance and Idlewood Boulevard. None of the entrances are signalized in this western segment of the corridor.

There are fewer entrances in the eastern segment of the corridor between the WWRC entrance and Barren Ridge Road. The intersection of US 250 at Barren Ridge Road is signalized.

### 3.1.2 Woodrow Wilson Avenue (VA 358)

Operational conditions examined for Woodrow Wilson Avenue were focused at the entrance to the WWRC Complex at the signalized intersection with US 250 .
Based on input from the stakeholders and public meeting participants, there are three existing issues at the intersection of US 250 and Woodrow Wilson Avenue:

1. The westbound right turn lane from US Route 250 onto Woodrow Wilson Avenue experiences extensive queues in the AM peak hour. With buses, faculty, students, parents, and office workers all arriving in a
 very short, condensed timeframe, the queue extends up the hill towards Barren Ridge Road.
2. The southbound approach on Woodrow Wilson Avenue to US Route 250 experiences extensive queues during AM and PM peak hours. Traffic leaving the site queues to the elementary school and often blocks access to the southbound left-turn lanes at the WWRC exit.
3. There are no secondary or emergency access points to the Complex.

### 3.1.3 Barren Ridge Road Intersection with US 250

The US 250 intersection at Barren Ridge Road is approximately 3,455-feet ( 0.65 -miles) to the east of the WWRC Complex entrance. The connection at US 250 is signalized, and the existing striping and pavement conditions are in poor condition. A new signal plan and equipment upgrades will be required should this location be selected for advancement as the long-term improvement option. Barren Ridge Road connects US 250 to surrounding agricultural and residential uses.

### 3.2 Historic Traffic Volumes

Historically, traffic on US 250 has experienced an annual growth average of 1.0\%. However, traffic volumes decreased during the recession years between 2007-2012. Westbound traffic on US 250, from I-81 to Idlewood Boulevard recovered to $90 \%$ of pre-recession volumes by 2016, however eastbound traffic from Idlewood Boulevard to Tinkling Springs Road has remained consistent since 2013, operating $21 \%$ below 2006 volumes. The slow return to pre-recession eastbound traffic volumes can be attributed to the 2016 opening of VA 636 (LifeCore Drive). LifeCore Drive is a new roadway that provides a direct link from the WWRC Complex to I-64, eliminating the need for traffic exiting the WWRC Complex to turn left onto US 250 and travel east to Tinkling Springs Road to connect to I-64. Table 2 provides the historic AADT for the US 250 corridor study area. US 250 historic AADT is shown in Table 1.

Table 1: Historic AADT US 250

| Year | US 250: I-81 to <br> Idlewood Boulevard | \% <br> Change | US 250: Idlewood Boulevard <br> to Tinkling Springs Road | \% <br> Change |
| :---: | :---: | :---: | :---: | :---: |
| 2006 | 20,000 | $*$ | 19,000 | $*$ |
| 2007 | 18,000 | -10 | 15,000 | -21 |
| 2008 | 18,000 | 0 | 14,000 | -7 |
| 2009 | 17,000 | -6 | 14,000 | 0 |
| 2010 | 16,000 | -6 | 13,000 | -7 |
| 2011 | 16,000 | 0 | 13,000 | 0 |
| 2012 | 16,000 | 13,000 | 15,000 | 15 |
| 2013 | 18,000 | -6 | 15,000 | 0 |
| 2014 | 17,000 | 6 | 15,000 | 0 |
| 2015 | 18,000 | 0 | 15,000 | 0 |
| 2016 | 18,000 |  |  | 0 |

Source: VDOT

Historic AADT data for Woodrow Wilson Avenue is not available, as the roadway is not included in the VDOT traffic count program. Traffic counts for Hornet Road, an internal roadway in the WWRC complex, were taken in 2007 and 2013. ADT for Hornet Road in 2007 were 1,600 vehicles in 2007, and 2,500 vehicles in 2013, an increase of $56 \%$. This increase can be attributed to the expansion of Wilson Middle and High Schools and the construction of Wilson Elementary School during the period between count years.

### 3.3 Current Traffic Volumes (2017)

VDOT conducted two-way 48-hour traffic counts at two locations on US 250 on February 22 and 23, 2017, during school operating hours, and 12-hour turning movement counts on February 23. Count station \#1 was located between Idlewood Boulevard and Woodrow Wilson Avenue; count station \#2 was located between Woodrow Wilson Boulevard and Barren Ridge Road. Turning movement counts were gathered at the intersections of US 250 with Barren Ridge Road, Woodrow Wilson Avenue, and Idlewood Boulevard.

Traffic counts show current volumes to be consistent with historic traffic volumes and growth patterns within the study area. AM peak hour use at the WWRC Complex entrance is between 7:15 and 8:15 AM; PM peak hour use occurs between 3:00-4:00 PM. The US 250 corridor weekday peak hour use is between 4:45 PM - 5:45 PM.

The peak hour operational time differs from peak hour turning movements due to an increase in vehicles entering and exiting the WWRC complex at times prescribed by school dismissal schedules versus normal corridor commuting times.

Current year US 250 study area ADT is summarized in Table 2. 2017 traffic counts at the WWRC entrance are summarized in Table 3. Map 8 shows count locations.

Table 2: US 250 Current ADT (2017)

|  | $\mathbf{2 / 2 2 / 2 0 1 7}$ | $\mathbf{2 / 2 3 / 2 0 1 7}$ | Average Daily Traffic |
| :--- | :--- | :--- | :--- |
| Station 1 | 19,527 | 20,187 | 19,857 |
| Station 2 | 15,487 | 16,428 | 15,958 |

Source: VDOT Staunton District Planning
Table 3: WWRC Complex Entrance Peak Hour Volumes

| Year | Peak Hour Entering Volume | Peak Hour Exiting Volume | Total |
| :--- | :--- | :--- | :--- |
| 2017 | 1,097 | 463 | 1,560 |

[^0]
### 3.3.1 Existing Level of Service/Capacity

Capacity analyses were performed to assess traffic conditions for each analysis scenario. The analysis included delay, level of service, and 95th percentile queuing. The intersections were analyzed using SYNCHRO Version 9.1 based on HCM 2010 methodologies with the following assumptions:

- The peak hour factor (PHF) for the overall intersection was obtained from the turning movement counts; Analysis used specific movement PHFs to accurately represent the condensed time frame of arriving and departing volumes.
- Heavy vehicle percentages for each movement based on the collected traffic data with a minimum percentage of $2 \%$;
- The existing signal timings (Synchro files) provided by VDOT; and
- All other software defaults remain unchanged.

The existing capacity analysis was performed based on the existing intersection geometrics and the existing peak hour counts described above.

Table 4 shows that the US Route 250/Woodrow Wilson Avenue/Lifecore Drive intersection operates at an overall Level of Service (LOS) E in the AM peak hour and D in the PM peak hour. Several of the individual turning movements operate at LOS E or F in both peak hours.

The westbound right turn operates at a LOS E with an average delay of 75.5 seconds/vehicle in the AM peak hour. The southbound though-right lane operates at a LOS F with an average delay of 95.2 seconds per vehicle in the AM peak hour and LOS E with an average delay of 58.2 seconds/vehicle in the PM peak hour.

## US 250/WWRC Small Area Study

Table 4: Existing Intersection Level of Service, Delay, and Queue Summary

| Intersection and Type of Control | Movement and Approach | Turn Lane Storage (ft) | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Delay ${ }^{1}$ (sec/veh) | $\underset{1}{\mathrm{LOS}}$ | $\begin{aligned} & \text { HCM } \\ & \text { 2000 } \\ & \text { 95th } \end{aligned}$ <br> Percentile Queue Length (ft) | Delay ${ }^{1}$ $(\mathrm{sec} / \mathrm{veh})$ | $\underset{1}{\mathrm{LOS}}$ | HCM <br> 2000 <br> 95th <br> Percentile Queue Length (ft) |
|  |  |  | 2017 Existing (195s cycle length) |  |  |  |  |  |
| 1. US Route 250 (E-W) at Lifecore Drive ( N ) <br> Woodrow Wilson Ave (S) | NB Left (2) | 220 | 100.7 | F | \#121 | 61.8 | E | 201 |
|  | NB Thru-Right |  | 103.9 | F | 449 | 54.2 | E | 118 |
|  | NB Approach |  | 103.0 | $F$ | -- | 58.9 | E | -- |
|  | SB Left (2) | 220 | 99.9 | F | 157 | 56.3 | E | 215 |
| Signalized | SB Thru-Right |  | 95.2 | F | 396 | 58.2 | E | 356 |
|  | SB Right |  | -- | -- | -- | -- | -- | -- |
|  | SB Approach |  | 97.5 | $F$ | -- | 57.1 | E | -- |
|  | EB Left | 482 | 55.7 | E | 402 | 33.2 | C | 81 |
|  | EB Thru (2) |  | 35.8 | D | 213 | 46.5 | D | 397 |
|  | EB Right | 490 | 29.5 | C | 1 | 26.2 | C | 36 |
|  | EB Approach |  | 41.9 | D | -- | 41.1 | D | -- |
|  | WB Left | 350 | 44.0 | D | 77 | 35.5 | D | 58 |
|  | WB Thru (2) |  | 56.6 | E | 294 | 47.6 | D | 361 |
|  | WB Right | 250 | 75.5 | E | 550 | 18.8 | B | 0 |
|  | WB Approach |  | 66.7 | E | -- | 41.3 | D | -- |
|  | Overall |  | 70.0 | E | -- | 48.7 | D | -- |

### 3.4 Crash Summary

61 crashes were recorded in the study area for the 5 -year period between January 1, 2012 and December 31, 2016. 46 recorded crashes occurred on US 250 between Barren Ridge Road and Idlewood Boulevard. 15 crashes occurred on Woodrow Wilson Avenue between the WWRC Complex entrance and Hornet Road.

The intersection of US 250 at Barren Ridge Road reported the highest concentration of crashes for the 5 -year period with 19 . The highest reported number of crashes, 21, occurred between 3:00 PM 6:00 PM. Fifty-three (87\%) recorded crashes occurred during months schools were in session.

Rear end crashes accounted for $53 \%$ of all crashes in the study area, with 23 rear-end collisions occurring on US 250, and 11 occurring on Woodrow Wilson Avenue. Angle crashes were the second most reported crash type in the study area, with a total of 22 crashes recorded for the 5 -year period.

32 crashes reported property damage only, 28 crashes involved an injury, and 1 fatality was recorded. Figure 7 illustrates the location and severity of crashes in the study area between 2012 and 2016.

Figure 7: Study Area Crashes (2012-2016)


## 4. Improvement Options and Analysis

Short- and long-term options were analyzed to identify improvements that best enhance corridor operations. Short-term options focus on improving operating conditions at the WWRC Complex entrance at the US 250/Woodrow Wilson intersection. Long-term options focus on identifying possible locations for a second entrance to the WWRC Complex.

### 4.1 Short-Term Improvement Options

Goals for the short-term improvement options for the US 250/Woodrow Wilson Avenue/Lifecore Drive intersection were:

- Alleviate congestion at the intersection;
- Minimize delay and queues during the AM and school PM peak hours; and
- Maintain existing multi-use path/pedestrian amenities.
- Currently there is a multi-use path along the western side of Woodrow Wilson Avenue and a crosswalk across Jefferson Highway. Maintaining these facilities was a priority in developing the short-term options.

The following short-term-options were developed and evaluated:

1. Channelized free-flow westbound right turn lane and associated northbound receiving lane;
2. Separate southbound right turn lane; and
3. Signal timing adjustments.

Short-term improvements are shown on Figure 8 (wide view) and Figure 9 (close view). Shortterm improvements do not include an analysis of internal traffic operations at the WWRC. Additional analysis will be required to support a project application should recommendations be advanced for funding consideration.

The recommended channelized westbound right turn lane and northbound receiving lane allow traffic entering the Complex to operate as a free-flow movement. This improvement includes a concrete triangular median on US 250 to separate the right turn from main-line traffic and an additional northbound lane on Woodrow Wilson Avenue to receive the right turning traffic.

The north-bound receiving lane extends approximately 1,200-feet from the WWRC Complex entrance and terminates at Wilson Elementary. An option to extend the receiving lane an additional 8oo-feet past Wilson Elementary to the future roundabout location at Hornet Road is provided in Appendix B-2. Further analysis of internal traffic movements at the WWRC Complex will be required to determine the benefits of extending the north-bound receiving lane to the future roundabout location and if extending the receiving lane impacts the future roundabout design.

Improvements for southbound traffic on Woodrow Wilson Avenue exiting the WWRC Complex include a separate southbound right-turn lane with 250 -feet of storage and 200 feet of taper is recommended at the WWRC Complex exit. The remaining southbound lanes exiting the WWRC Complex consist of one through traffic lane, and dual left-turn lanes. This improvement will require shifting the existing lanes west and relocating the existing multi-use path.

Short-term improvements also require modifications to the existing traffic signal timings at the WWRC Complex entrance intersection.

## US 250/WWRC Small Area Study

Figure 8: Short-term Improvements (Wide View)


WOODROW WILSON SMALL AREA STUDY
Proposed Short Term Improvements - January 2018

## US 250/WWRC Small Area Study

Figure 9: Short-term Improvements (Close-up View)


WOODROW WILSON SMALL AREA STUDY
Proposed Short Term Improvements - January 2018
NOT TO SCALE

### 4.1.1 Short-Term Improvements Capacity Analysis

The impacts of the short-term improvements were analyzed under both existing conditions (2017) and future conditions (2022).

The results of the existing conditions (2017) analysis are summarized in Table 5, and the analysis worksheets are contained in Appendix B.

Table 5 shows with the short-term improvements and 2017 traffic volumes the overall LOS of the intersection will improve in both the AM and PM peak hours. The free-flow westbound right turn lane eliminates the delay and queue for that movement and allows for signal timing adjustments to improve the operations of other movements. The southbound right queue is reduced to 177 feet and will be contained within the storage area of the new turn lane. The southbound right delay is reduced by 30 seconds/vehicle.

The 2022 traffic volumes were developed using the following annual linear growth rates (applied to the existing 2017 volumes):

- US Route 250: 1.0\%;
- Woodrow Wilson Avenue: 0.5\%; and
- Lifecore Drive 2.0\%

The capacity of the intersection under 2022 traffic conditions was analyzed for both no-build (no short-term improvements) and build conditions (with the short-term improvements). The results of the analysis are summarized in Table 6.

Table 6 shows that without the short-term improvements, the operations and queueing of the intersection will deteriorate in 2022.

With the short-term improvements, the overall intersection LOS will improve in both the AM and PM peak hours. The free-flow westbound right turn lane, again, eliminates the delay and queue for that movement and allows for signal timing adjustments to improve the operations of other movements. The southbound right queue is reduced to 160 feet and will be contained within the storage area of the new turn lane. The southbound right delay is reduced by over 40 seconds/vehicle.

Table 5: Existing Intersection Operational Conditions with Short-Term Improvements

| Intersection and Type of Control | Movement and Approach | Turn Lane Storage (ft) | AM Peak Hour |  |  | PM Peak Hour |  |  | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Delay ${ }^{1}$ (sec/veh) | $\operatorname{LOS}^{1}$ | $\begin{aligned} & \text { HCM } \\ & 2000 \\ & 95 \text { th } \end{aligned}$ <br> Percentile Queue Length (ft) | Delay ${ }^{1}$ (sec/veh) | $\operatorname{LOS}^{1}$ | HCM <br> 2000 <br> 95th <br> Percentile <br> Queue <br> Length <br> (ft) | $\begin{gathered} \text { Delay }{ }^{1} \\ \text { (sec/veh) } \end{gathered}$ | $\operatorname{LOS}^{1}$ | HCM 2000 95th <br> Percentile Queue Length (ft) | $\begin{gathered} \text { Delay }{ }^{1} \\ \text { (sec/veh) } \end{gathered}$ | $\operatorname{LOS}^{1}$ | HCM <br> 2000 <br> 95th <br> Percentile <br> Queue <br> Length <br> (ft) |
|  |  |  | Existing Lane Use |  |  |  |  |  | With Free-flow WBR Turn Lane and Exclusive SBR |  |  |  |  |  |
| 1. US Route 250 (E-W) at Lifecore Drive ( N ) Woodrow Wilson Ave (S) | NB Left (2) | 220 | 100.5 | F | \#121 | 63.9 | E | 201 | 83.6 | F | 119 | 56.1 | E | 185 |
|  | NB Thru-Right |  | 103.2 | F | 449 | 56.8 | E | 118 | 87.4 | F | 442 | 56.6 | E | 114 |
|  | NB Approach |  | 102.4 | $F$ | -- | 61.2 | E | -- | 86.3 | $F$ | -- | 56.3 | E | -- |
|  | SB Left (2) | 220 | 100.3 | F | 153 | 57.3 | E | 214 | 84.0 | F | 151 | 51.1 | D | 200 |
| Signalized | SB Thru-Right |  | 97.6 | F | 395 | 61.8 | E | 356 | - | - | - | - | - | - |
|  | SB Thru |  | - | - | - | - | - | - | 50.1 | D | 165 | 41.2 | D | 161 |
|  | SB Right | 250 | - | - | - | - | - | - | 22.2 | C | 90 | 33.5 | C | 177 |
|  | SB Approach |  | 98.9 | F | -- | 59.2 | E | -- | 57.2 | E | -- | 45.1 | D | -- |
|  | EB Left | 482 | 53.8 | D | 398 | 31.8 | C | 81 | 56.7 | E | 440 | 29.5 | C | 77 |
|  | EB Thru (2) |  | 35.3 | D | 211 | 45.8 | D | 398 | 37.9 | D | 220 | 43.2 | D | 375 |
|  | EB Right | 490 | 29.0 | C | 1 | 26.2 | C | 36 | 29.8 | C | 1 | 23.8 | C | 33 |
|  | EB Approach |  | 40.9 | D | -- | 40.5 | D | -- | 43.1 | D | -- | 37.9 | D | -- |
|  | WB Left | 350 | 43.4 | D | 76 | 34.8 | D | 58 | 53.5 | D | 80 | 32.5 | C | 55 |
|  | WB Thru (2) |  | 55.8 | E | 292 | 47.5 | D | 361 | 71.3 | E | 339 | 45.0 | D | 345 |
|  | WB Right | 250 | 75.0 | E | 549 | 18.8 | B | 0 | Free-Flow ${ }^{(2)}$ |  |  | Free-Flow ${ }^{(2)}$ |  |  |
|  | WB Approach |  | 66.1 | E | -- | 41.2 | D | -- | 29.9 | C | -- | 35.6 | D | -- |
|  | Overall |  | 69.6 | E | -- | 49.5 | D | -- | 46.9 | D | -- | 42.2 | D | -- |

## US 250/WWRC Small Area Study

Table 6: Future Intersection Operational Conditions Summary with Short-Term Improvements

| Intersection and Type of Control | Movement and Approach | Turn <br> Lane Storage <br> (ft) | AM Peak Hour |  |  | PM Peak Hour |  |  | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Delay ${ }^{1}$ (sec/veh) (sec/veh) | $\underset{1}{\operatorname{LOS}}$ | HCM <br> 2000 <br> 95th <br> Percentile <br> Queue <br> Length <br> (ft) | Delay ${ }^{1}$ (sec/veh) | $\underset{1}{\operatorname{LOS}}$ | HCM 2000 95 th <br> Percentile Queue Length (ft) | Delay ${ }^{1}$ (sec/veh) | $\underset{1}{\mathrm{LOS}}$ | HCM <br> 2000 <br> 95th <br> Percentile <br> Queue <br> Length <br> (ft) | $\begin{gathered} \text { Delay }{ }^{1} \\ \text { (sec/veh) } \end{gathered}$ | $\underset{1}{\mathrm{LOS}}$ | HCM 2000 95 th <br> Percentile Queue Length (ft) |
|  |  |  | 2022 No Build (existing 195s cycle length) |  |  |  |  |  | With Free-flow WBR Turn Lane and SBR Turn Lane (optimized 150s cycle length) |  |  |  |  |  |
| 1. US Route 250 (E-W) at Lifecore Drive ( N ) | NB Left (2) | 220 | 107.3 | F | \#137 | 65.2 | E | 224 | 70.5 | E | 100 | 56.8 | E | 181 |
|  | NB ThruRight |  | 108.6 | F | \#472 | 56.3 | E | 121 | 94.9 | F | \#404 | 57.0 | E | 104 |
| Woodrow Wilson Ave (S) | $\begin{array}{r} N B \\ \text { Approach } \end{array}$ |  | 108.2 | F | -- | 61.9 | E | -- | 87.8 | F | -- | 56.9 | $E$ | -- |
|  | SB Left (2) | 220 | 108.8 | F | 162 | 58.8 | E | 233 | 85.4 | F | 127 | 52.5 | D | 188 |
| Signalized | SB ThruRight |  | 104.5 | F | 414 | 60.8 | E | 373 | 45.6 | D | 145 | 41.2 | D | 153 |
|  | SB Right |  | -- | -- | -- | -- | -- | -- | 21.3 | C | 70 | 34.8 | C | 160 |
|  | SB Approach |  | 106.6 | $F$ | -- | 59.6 | E | -- | 57.1 | E | -- | 46.2 | D | -- |
|  | EB Left | 482 | 61.3 | E | 412 | 34.7 | C | 91 | 66.6 | E | 392 | 30.2 | C | 71 |
|  | EB Thru (2) |  | 36.0 | D | 222 | 48.1 | D | 442 | 35.5 | D | 197 | 41.9 | D | 355 |
|  | EB Right | 490 | 29.7 | C | 0 | 26.7 | C | 36 | 27.3 | C | 6 | 23.0 | C | 33 |
|  | EB Approach |  | 44.1 | D | -- | 42.4 | D | -- | 45.2 | D | -- | 36.9 | D | -- |
|  | WB Left | 350 | 45.9 | D | 80 | 36.7 | D | 67 | 26.1 | C | 69 | 29.7 | C | 53 |
|  | WB Thru (2) |  | 59.2 | E | 311 | 48.7 | D | 392 | 66.9 | E | 283 | 41.5 | D | 313 |
|  | WB Right | 250 | 94.2 | F | 604 | 18.7 | B | 0 | 1.1 | A | 0 | 0.1 | A | 0 |
|  | Approach |  | 78.2 | $E$ | -- | 42.2 | D | -- | 27.0 | C | -- | 33.0 | C | -- |
|  | Overall |  | 77.0 | E | -- | 50.5 | D | -- | 46.9 | D | -- | 41.7 | D | -- |

### 4.2 Long-Term Improvement Options

While the short-term analysis and recommendations focus on improving operational and safety conditions at the entrance to the WWRC Complex, a single entrance to the WWRC Complex will not adequately serve the long-term traffic demands in the study area or address concerns raised by stakeholders regarding the need for a second entrance to the WWRC Complex. Long-term improvement options focus on identifying a second entrance location to the WWRC Complex.

To address long-term improvement options, the study team developed the following goals for a second entrance location:

1. Alleviate congestion at the existing entrance;
2. Provide secondary and emergency access to the Complex; and
3. Provide access to adjacent/undeveloped parcels.

To accomplish the long-term goals, the construction of a new road that connects to the surrounding roadway network is required. Based on traffic projections and nature of the proposed road, a typical section was developed with the following construction specifications:

- Two 13' vehicular travel lanes (one lane in each direction);
- 10 ' multi-use path on one side; and
- Option for either curb and gutter or ditch section.

Three potential entrance locations were identified to determine planning level cost estimates. Since the options include new or modified connections to US 250, the analysis includes each option's respective connection point, the US 250/Woodrow Wilson Avenue/Lifecore Drive intersection, and the US 250/Barren Ridge Road/Mule Academy Road intersection. For purposes of analysis, traffic signals were assumed at the connection points on US 250 for Options A and B. Long-term options are conceptual and represent planning level efforts; engineering, design, and environmental studies have not been performed. Additionally, analysis of internal WWRC traffic will be required should a long-term option be advanced in the future. Long-term entrance options are detailed below and are shown on Figure 10.

## Option A

Option A is located west of Woodrow Wilson Avenue connecting US 250 near the Virginia Employment Commission building with the western portion of the Complex. The option is approximately 3,700 feet in length and would connect with the Complex via Hornet Road near the Rehabilitation Center. This option provides direct access to the Complex and relief for the eastbound left movement in the AM peak hour and the southbound right in both the AM and PM peak hours. Most of the right-of way for the alignment as shown is across State-owned property.

Option A alone does not provide relief for the heavy westbound traffic in the AM peak hour, but the congestion related to this westbound traffic is generally resolved with the short-term improvements at US 250 and Woodrow Wilson Avenue/Lifecore Drive.

Right and left turn lanes on US 250 would likely be required with Option A; however, a more detailed analysis of intersection control will be needed if this option were pursued for funding.

## Option B

Option B is located east of Woodrow Wilson Avenue, connecting to US 250 near the crest of the hill, and connects into the Complex behind the Technical Center near the current bus parking area. The option is approximately 3,900 feet in length and provides access to the Middle/High School portion of the Complex. This option provides direct access to the Complex and provides relief for the westbound traffic in the AM peak hour and the southbound left traffic in both the AM and PM peak hours. Implementing this option will provide access to adjacent/undeveloped parcels but requires obtaining right-of-way from individual private owners.

Right and left turn lanes on US 250 would likely be required with Option B; however, a more detailed analysis of intersection control will be needed if this option is pursued for funding.

## Option C

Option C connects to Barren Ridge Road and the existing traffic signal at the US 250/Barren Ridge Road/Mule Academy Road intersection and connects into the Complex behind the Technical Center near the current bus parking area. The option is approximately 5,000 feet in length and provides access to the Middle/High School portion of the Complex.

Barren Ridge Road requires realignment to become the minor street where southbound traffic yields to through traffic on the proposed road. The portion of Barren Ridge Road between Option C and US 250 requires widening from one (1) to two (2) lanes to accommodate a southbound left turn lane. In addition, westbound right and eastbound left turn lanes would be added/extended on US 250. The existing traffic signal would be modified to accommodate the improvements.

Option C provides direct access to the Complex, and relief for the westbound traffic in the AM peak hour and the southbound left traffic in both the AM and PM peak hours. The option provides access to adjacent/undeveloped parcels but requires obtaining right-of-way from individual private owners.

### 4.2.1 Long-Term Improvements Capacity Analysis

The long-term improvement options capacity analysis used 2040 forecasted traffic volumes. To develop the 2040 traffic volumes, the following annual linear growth rates were applied to existing traffic volumes:

1. US 250: $1.0 \%$;
2. Woodrow Wilson Avenue: 0.5\%;
3. Lifecore Drive 2.0\%.

Analysis of future operating conditions show Options A - C provide relief for the existing WWRC Complex entrance. Table 4 summarizes the future traffic volume analysis. Analysis worksheets are contained in Appendix C.

The following summarizes the traffic diversion assumptions for each option:

## Option A

- Expect up to $60 \%$ of the existing EB left turns to divert to Option A. This amounts to 183 lefts in the AM peak hour. The diversions would most likely come from traffic heading towards the middle/high school area of the campus.
- Expect up to 70\% of the existing SB right turns to divert to Option A. This amounts to 147 rights in the school PM peak hour.


## Option B

- Expect up to $60 \%$ of the existing WB right turns and $20 \%$ of the existing NB thru traffic to divert to Option B. This amounts to 346 rights in the AM peak hour. The diversions would most likely come from traffic heading towards the middle/high school area of the campus.
- Expect up to $50 \%$ of the existing SB left turns to divert to Option B. This amounts to 159 lefts in the school PM peak hour.


## Option C

- Up to $60 \%$ of the existing WB right turns and $20 \%$ of the existing NB through traffic to divert to Option C. This amounts to 346 rights in the AM peak hour. The diversions would most likely come from traffic heading towards the middle/high school area of the campus.
- Expect up to $50 \%$ of the existing SB left turns to divert to Option C. This amounts to 159 lefts in the school PM peak hour.


## US 250/WWRC Small Area Study

Figure 10: Long-term Secondary Access Options


## Table 7: Future Intersection Operating Conditions Summary with Long Term Improvement




## 5. Cost Estimates

### 5.1 Short Term Cost Estimates

Planning level cost estimates developed for the short-term improvements range between $\$ 1.5$ to $\$ 1.8$ million (2018 dollars). Estimates include engineering, construction, right-of-way, utilities, traffic signal work, relocation of the pedestrian path, and contingency fees. Detailed estimates are included in Appendix B. Short-term improvements are shown on Figure 7 (close view) and Figure 8 (wide view).

The short-term cost estimate increases to $\$ 2.1$ to $\$ 2.5$ million (2018 dollars) if the northbound receiving lane on Woodrow Wilson Avenue is extended to the future roundabout at Hornet Road.

### 5.2 Long Term Improvement Options Cost Estimates

Planning level cost estimates developed for the long-term improvements identified in Section 4.2 include engineering, construction, right-of-way, utilities, traffic signal work, and contingency fees. Given the extended timeline for construction, the estimates are inflated to 2026 dollars. Planning level cost estimates are summarized below Figure 10 illustrates long-term secondary/emergency access options A - C. Detailed cost estimates for each can be found in Appendix C.

- Option A: $\$ 13.3$ to $\$ 16.1$ million in 2026 dollars;
- Option B: \$13.3 to $\$ 16.6$ million in 2026 dollars; and
- Option C: $\$ 19.6$ to $\$ 23.7$ million in 2026 dollars.


## Detailed cost estimates are included in Appendix C.

## 6. References

USDA-NRCSa. 2015. Official Soil Series Descriptions (OSD) with series extent mapping capabilities. Available at: http://soils.usda.gov/technical/classification/osd/index.html.

Woods, A.J., J.M. Omernik, and D.D. Brown. 1999. Level III and IV Ecoregions of Delaware, Maryland, Pennsylvania, Virginia, and West Virginia. USEPA: Corvallis, Oregon. Available at: ftp://ftp.epa.gov/wed/ecoregions/reg3/reg3 eco desc.doc.

## Appendix A:

## Public Participation/Outreach



Wednesday January 31, 2018
4:00 PM to 6:00 PM
Wilson Middle School Cafeteria
232 Hornet Road, Fishersville, VA
The Staunton-Augusta-Waynesboro MPO is conducting a transportation study of US 250 in the vicinity of the Wilson Workforce and Rehabilitation Center in an effort to improve access and safety. Study recommendations will be used to guide future transportation improvements in the area. Come out and give us your input!

Let's work together to keep the area safe, vibrant, and moving!


For additional information please contact Scott Philips at 885-5174 or by email at scott@cspdc.org

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| STAUNTON VA 24401 | Net Amt: | \$179.08 |  |  |
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# Have something to say about traffic around Wilson schools? 



Often parents have to choose whether to arrive an hour early to pick up students or to walt an hour in line once school lets out, not to mention those rehab center employees who need to get In and out of the complex

## ADVERTISING

ReplayAn effective solution to the Woodrow Wilson complex traffic issue has eluded officials, but a panel next week will hopefully provide some answers

The Staunton-Augusta-Waynesboro Metropolitan Planning Organization will be hosting a public open house and information meeting for the Wilson Workforce and Rehabilitation Center Transportation Study on Thursday. The forum will take place at Wilson Middle School from 5 p.m. to 7 p.m.

The study will examine alterative and secondary access locations to the Woodrow Wilson complex, access management along U.S. 250, and operational and safety issues along the corridor with the intent of developing short- and long-term improvement recommendations, according to a press release.

Study recommendations will be used to guide future transportation Improvements In the area. The public is Invited to attend the open house and provide input.

For more Information about the forum, contact Bonnie Rledesel, 540-885-5174, bonnle@raspdc.org or Scott Philips, 540-885-5174, scott@cspdc.org.

More In the news: Wilson senior finds passion in helping others (/story/entertainment/2017/05/23/wilson-senior-finds-passion-helpingothers/3887570010


Wednesday January 31, 2018
4:00 PM to 6:00 PM
Wilson Middle School Cafeteria
232 Hornet Road, Fishersville, VA
The Staunton-Augusta-Waynesboro MPO is conducting a transportation study of US 250 in the vicinity of the Wilson Workforce and Rehabilitation Center in an effort to improve access and safety. Study recommendations will be used to guide future transportation improvements in the area. Come out and give us your input!

Let's work together to keep the area safe, vibrant, and moving!


For additional information please contact Scott Philips at 885-5174 or by email at scott@cspdc.org

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SAWMPO NOTICE Of PUBLIC MEETING

The Staunton-Augusta-Waynesboro Metropolitan Planning Organization
(SAWMPO) will hoidd a second and final public meeting on the transportation study of the US250 corridor near the WWRC entrance and the transportatio WWRC complex. The meeting will be held on January 31, 2018, from 4 -6 P.m., at the Wilson Middle School Cafeteria, 232 Hornet Road, Fishersville, A. Members of the public are encouraged to attend to learn about the study. Information atiout the
mall-area-transportation-study.
The SAWMPO ensures nondiscrimination and equal employment in all pro
grams and activities in accordance with Title VI and Title VII of the Civil Right
grams and activities in accordance with Title VI and Tite VII of the Civil Right
Act of 1964. If you have questions or concerns about your civil rights regard
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ing this project, or it special assistance for persons with disabilitites or limited
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English proficiency is needed, please contact the SAWMPO at 540-885-5174
Sign language or non-English, language interpreters will be provided if needed
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## APPENDIX B:

## Short-Term Improvements Detailed Cost Estimates

| PROJECT BUDGET: SHORT TERM IMPROVEMENTSPROPOSED PUBLIC ROAD CONSTRUCTIONWESTBOUND FREE-FLOW RIGHT TURN LANE FROM US ROUTE 250TO WOODROW WILSON AVENUEOPTION 1: END RECEIVING LANE AT EXISTING SCHOOL TURN LANE1/26/2018WESTBOUND RIGHT TURN LANE ON US ROUTE 250 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  | W |  | GH |
| 400' of Turn Lane Improvements (Pavement) at \$425/LF | \$ | 170,000 | \$ | 170,000 |
| Traffic Signal Pole Relocation | \$ | 100,000 | \$ | 100,000 |
| Construction Total A: | \$ | 270,000 | \$ | 270,000 |
| Right of Way and Utilities (35\% to 65\% Per VDOT formulas) | \$ | 94,500 | \$ | 175,500 |
| 25\% Contingency and PE Fee | \$ | 67,500 | \$ | 67,500 |
| Sub-Total A: | \$ | 432,000 | \$ | 513,000 |
| ADDITIONAL RECEIVING LANE ON WOODROW WILSON TO SCHOOL ENTRANCE |  |  |  |  |
| 1,200' of Additional Lane (Pavement) at \$400/LF | \$ | 480,000 | \$ | 480,000 |
| Construction Total B: | \$ | 480,000 | \$ | 480,000 |
| Right of Way and Utilities (35\% to 65\% Per VDOT formulas) | \$ | 168,000 | \$ | 312,000 |
| 25\% Contingency and PE Fee | \$ | 120,000 | \$ | 120,000 |
| Sub-Total B: | \$ | 768,000 | \$ | 912,000 |
| SOUTHBOUND RIGHT TURN LANE ON WOODROW WILSON AVE |  |  |  |  |
| 450' of Turn Lane Improvements (Pavement) at \$425/LF | \$ | 191,250 | \$ | 191,250 |
| Pedestrian Pole Relocation | \$ | 15,000 | \$ | 15,000 |
| 500' of Multi-Use Path Relocation at \$45/LF | \$ | 22,500 | \$ | 22,500 |
| Construction Total C: | \$ | 228,750 | \$ | 228,750 |
| Right of Way and Utilities (35\% to 65\% Per VDOT formulas) | \$ | 80,063 | \$ | 148,688 |
| 25\% Contingency and PE Fee | \$ | 57,188 | \$ | 57,188 |
| Sub-Total C: | \$ | 366,001 | \$ | 434,626 |
| Total Bid Items A + + C Rounded (2018 Dollars): | \$ | 1,566,000 | \$ | 1,859,600 |
| Total Bid Items A + + C Rounded (2026 Dollars): | \$ | 1,983,800 | \$ | 2,355,700 |

[^1]| PROJECT BUDGET: SHORT TERM IMPROVEMENTS PROPOSED PUBLIC ROAD CONSTRUCTION <br> WESTBOUND FREE-FLOW RIGHT TURN LANE FROM US ROUTE 250 TO WOODROW WILSON AVENUE <br> OPTION 2: END RECEIVING LANE AT ROUNDABOUT $1 / 26 / 2018$ <br> WESTBOUND RIGHT TURN LANE ON US ROUTE 250 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 400' of Turn Lane Improvements (Pavement) at \$425/LF | \$ | 170,000 | \$ | 170,000 |
| Traffic Signal Pole Relocation | \$ | 100,000 | \$ | 100,000 |
| Construction Total A: | \$ | 270,000 | \$ | 270,000 |
| Right of Way and Utilities (35\% to 65\% Per VDOT formulas) | \$ | 94,500 | \$ | 175,500 |
| $25 \%$ Contingency and PE Fee | \$ | 67,500 | \$ | 67,500 |
| Sub-Total A: | \$ | 432,000 | \$ | 513,000 |


| ADDITIONAL RECEIVING LANE ON WOODROW WILSON TO ROUNDABOUT |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
|  |  |  |  |  |
| 2,000' of Additional Lane (Pavement) at \$400/LF | $\$$ | 800,000 | $\$$ | 800,000 |
| Construction Total B: | $\$$ | 800,000 | $\$$ | 800,000 |
| Right of Way and Utilities (35\% to 65\% Per VDOT formulas) | $\$$ | 280,000 | $\$$ | 520,000 |
| 25\% Contingency and PE Fee | $\$$ | 200,000 | $\$$ | 200,000 |
| Sub-Total B: | $\mathbf{\$}$ | $\mathbf{1 , 2 8 0 , 0 0 0}$ | $\mathbf{\$}$ | $\mathbf{1 , 5 2 0 , 0 0 0}$ |

## SOUTHBOUND RIGHT TURN LANE ON WOODROW WILSON AVE

| 450' of Turn Lane Improvements (Pavement) at \$425/LF | \$ | 191,250 | \$ | 191,250 |
| :---: | :---: | :---: | :---: | :---: |
| Pedestrian Pole Relocation | \$ | 15,000 | \$ | 15,000 |
| 500' of Multi-Use Path Relocation at \$45/LF | \$ | 22,500 | \$ | 22,500 |
| Construction Total C: | \$ | 228,750 | \$ | 228,750 |
| Right of Way and Utilities (35\% to 65\% Per VDOT formulas) | \$ | 80,063 | \$ | 148,688 |
| 25\% Contingency and PE Fee | \$ | 57,188 | \$ | 57,188 |
| Sub-Total C: | \$ | 366,001 | \$ | 434,626 |
| Total Bid Items A + B + C Rounded (2018 Dollars): | \$ | 2,078,000 | \$ | 2,467,600 |
| Total Bid Items A + B + C Rounded (2026 Dollars): | \$ | 2,632,300 | \$ | 3,125,900 |

[^2]APPENDIX C:
Long-Term Improvements Options A - C Sections, Detailed Cost Estimates


[^3]
## PROJECT BUDGET - DETAILED BREAKDOWN FOR OPTION A PROPOSED PUBLIC ROAD CONSTRUCTION ROUTE 250 (JEFFERSON HWY) TO SITE (APPROX. 3,700 LF)



| PROJECT BUDGET: OPTION BPROPOSED PUBLIC ROAD CONSTRUCTIONROUTE 250 (JEFFERSON HWY) TO SITE (APPROX. 3,900 LF)1/26/2018 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| CONSTRUCTION |  |  |  |  |  |
|  |  | LOW |  | HIGH |  |
| Pavement Demolition and Resurfacing, Saw Cut, and Earthwork | \$ | 2,880,940 | \$ | 2,880,940 |  |
| Storm Sewer and Hydraulics | \$ | 1,348,275 | \$ | 1,348,275 |  |
| Pavement and Stone | \$ | 896,100 | \$ | 896,100 |  |
| Curb, Pavement Markings, Misc. | \$ | 252,500 | \$ | 252,500 |  |
| Retaining Wall | \$ | 112,500 | \$ | 112,500 |  |
| Maintenance of Traffic | \$ | 30,000 | \$ | 30,000 |  |
| Erosion and Sediment Control / Seeding | \$ | 150,000 | \$ | 150,000 |  |
| Traffic Signal | \$ | 350,000 | \$ | 350,000 |  |
| Sub-Total A: | \$ | 6,020,315 | \$ | 6,020,315 |  |
| OTHER CONSTRUCTION BID COSTS |  |  |  |  |  |
| Mobilization for Sub-Total A (Calculated per VDOT formulas) | \$ | 331,016 | \$ | 331,016 |  |
| Materials Testing | \$ | 120,406 | \$ | 120,406 |  |
| Construction Staking / Surveying (2\%) | \$ | 120,406 | \$ | 120,406 |  |
| Right of Way and Utilities (35\% to 65\% Per VDOT formulas) | \$ | 2,107,110 | \$ | 3,913,205 |  |
| Sub-Total B: | \$ | 2,678,939 | \$ | 4,485,033 |  |
| Total Bid Items ( $\mathbf{A}+\mathbf{B}$ ): | \$ | 8,699,254 | \$ | 10,505,348 |  |
| Contingency for Construction and PE (25\% of Total Bid Items) | \$ | 2,174,813 | \$ | 2,626,337 |  |
| Sub-Total C (Total Bid Items + Contingency): | \$ | 10,874,067 | \$ | 13,131,685 | CN |
| TOTAL PROJECT BUDGET ( C) (ROUNDED) in 2018 dollars: | \$ | 10,874,100 | \$ | 13,131,700 |  |
| TOTAL PROJECT BUDGET ( C) (ROUNDED) in 2026 dollars: | \$ | 13,775,000 | \$ | 16,634,800 |  |

*Inflation 3\% compounded annually for 8 years (3\%) to get from 2018 to 2026

* The total construction dollars above only includes items listed under the associated detailed breakdown.


## PROJECT BUDGET - DETAILED BREAKDOWN FOR OPTION B PROPOSED PUBLIC ROAD CONSTRUCTION ROUTE 250 (JEFFERSON HWY) TO SITE (APPROX. 3,900 LF)

| ENGINEER'S OPINION OF PROBABLE COSTS |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Item Code | Spec. No. | Description | Quantity | Unit | Unit Price |  | Total |
| Pavement Demolition and Resurfacing, Saw Cut, and Earthwork |  |  |  |  |  |  |  |
| 00110 | 301 | Clearing and Grubbing and Site Preparation | 8 | AC | \$ 10,000.00 | \$ | 80,000.00 |
| 10628 | 515 | Flexible Pavement Planing (0" - 3 " and variable) | 670 | SY | 7.00 | \$ | 4,690.00 |
| 24430 | 508 | Demolition of Pavement (Flexible) | 25 | SY | 10.00 | \$ | 250.00 |
| 24420 | 508 | Demolition of Pavement (Rigid) |  | SY | 25.00 | \$ |  |
| N/A | 508 | Building Demolition |  | LS | \$ 100,000.00 | \$ |  |
| 51910 | 315 | Saw Cut Existing Pavement |  | LF | \$ 5.00 | \$ |  |
|  | 303 | NS Undercut Excavation | 3000 | CY | 13.00 | \$ | 39,000.00 |
|  | 303 | NS Undercut Excavation Backfill | 3000 | CY | \$ 20.00 | \$ | 60,000.00 |
|  | 303 | Rock Excavation | 3000 | CY | \$ 100.00 | \$ | 300,000.00 |
|  | 303 | Excess Excavation | 19000 | CY | 15.00 | \$ | 285,000.00 |
| 00120 | 303 | Regular Excavation | 24000 | CY | 13.00 | \$ | 312,000.00 |
| 00140 | 303 | Borrow Excavation | 90000 | CY | \$ 20.00 | \$ | 1,800,000.00 |
| Sub-Total for Pavement Demolition and Resurfacing, Saw Cut, and Earthwork: $\$$ |  |  |  |  |  |  |  |
| Storm Sewer and Hydraulics |  |  |  |  |  |  |  |
| 01186 | 302 | 18" Reinforced Concrete Pipe | 975 | LF | \$ 100.00 | \$ | 97,500.00 |
| 01246 | 302 | 24" Reinforced Concrete Pipe | 1850 | LF | \$ 115.00 | \$ | 212,750.00 |
| 01306 | 302 | 30" Reinforced Concrete Pipe | 975 | LF | \$ 125.00 | \$ | 121,875.00 |
| 06815 | 302 | Drop Inlet DI-3A | 4 | EA | \$ 4,000.00 | \$ | 16,000.00 |
| 06818 | 302 | Drop Inlet DI-3B, $\mathrm{L}=6^{\prime}$ | 17 | EA | \$ $4,750.00$ | \$ | 80,750.00 |
| 06819 | 302 | Drop Inlet Dl-3B, L= ${ }^{\prime}$ | 17 | EA | \$ 5,000.00 | \$ | 85,000.00 |
| 06835 | 302 | Drop Inlet Di-3C, $\mathrm{L}=6^{\prime}$ | 2 | EA | \$ 5,000.00 | \$ | 10,000.00 |
| 06836 | 302 | Drop Inlet D-3C, $\mathrm{L}=8^{\prime}$ | 2 | EA | \$ 5,200.00 | \$ | 10,400.00 |
| N/A | N/A | Major Structure (Triple Box Culvert) | 1 | LS | \$ 350,000.00 | \$ | 350,000.00 |
| N/A | N/A | SWM Basins | 1 | LS | \$ 300,000.00 | \$ | 300,000.00 |
| 00588 | 501 | Underdrain UD-4 | 8000 | LF | \$ 8.00 | \$ | 64,000.00 |
| 00596 | 302 | Endwall EW-12 |  | EA | \$ 475.00 | \$ |  |
|  | Sub-Total for Storm Sewer and Hydraulics: |  |  |  |  | \$ | 1,348,275.00 |
| Pavement and Stone |  |  |  |  |  |  |  |
| 10607 | 211,315 | Asphalt Concrete Type SM-12.5A | 1830 | TON | \$ 125.00 | \$ | 228,750.00 |
|  |  | New Pavement | 1290 |  |  |  |  |
|  |  | New Shared Use Path | 480 |  |  |  |  |
|  | Overlay |  | 60 |  |  |  |  |
|  |  |  |  |  |  |  |  |
| 10611 | 211,315 | Asphalt Concrete Type IM-19.0D | 1290 | TON | \$ 115.00 | \$ | 148,350.00 |
|  |  |  |  |  |  |  |  |
| 10643 | 211,315 | Asphalt Concrete Type BM-25.0A | 2580 | TON | \$ 105.00 | \$ | 270,900.00 |
|  |  |  |  |  |  |  |  |
| 10128 | 208,309 | Aggregate Base Material Type 1, No. 21B | 8270 | TON | \$ 30.00 | \$ | 248,100.00 |
|  |  | New Pavement | 6000 | TON |  |  |  |
|  |  | New Shared Use Path | 1650 | TON |  |  |  |
|  |  | Underdrain | 620 | TON |  |  |  |
|  |  |  |  |  |  |  |  |
| Sub-Total for Pavement and Stone: |  |  |  |  |  | \$ | 896,100.00 |
| Curb, Sidewalk, Pavement Markings, Misc. |  |  |  |  |  |  |  |
| 12600 | 502 | Std. Combination Curb and Gutter CG-6 | 7800 | LF | \$ 25.00 | \$ | 195,000.00 |
|  |  | Permanent Signage | 1 | LS | \$ 5,000.00 | \$ | 5,000.00 |
| 54032 | 246 | Type B Class I Pavement Line Marking 4" | 7800 | LF | 2.00 | \$ | 15,600.00 |
| 54034 | 246 | Type B Class I Pavement Line Marking 6" |  | LF | \$ 3.00 | \$ |  |
| 54036 | 246 | Type B Class I Pavement Line Marking 8" |  | LF | \$ 4.00 | \$ |  |
| 54042 | 246 | Type B Class I Pavement Line Marking 24" | 100 | LF | \$ 15.00 | \$ | 1,500.00 |
| N/A | N/A | NS Yield Markings (Shark Teeth) |  | LF | \$ 15.00 | \$ |  |
| 54300 | 246 | Pavement Message Marking Elongated Arrow Single | 4 | EA | \$ 300.00 | \$ | 1,200.00 |
| 54310 | 246 | Pavement Message Marking Elongated Arrow Double | 4 | EA | \$ 400.00 | 5 | 1,600.00 |
| 13323 | 221,505 | Guardrail GR-MGS1 | 500 | LF | \$ 30.00 | S | 15,000.00 |
| N/A | N/A | Guardrail Terminal GR-MGS2 | 4 | EA | \$ 3,500.00 | 5 | 14,000.00 |
| N/A | N/A | NS Guardrail Terminal Site Preparation Minor | 4 | EA | \$ 750.00 | 5 | 3,000.00 |
| 13345 | 221,505 | Aggregate Base Material Ty. I or II No. 21A or 21B | 20 | TON | \$ 30.00 | \$ | 600.00 |
|  |  | Sub-Tota | Sidewalk | vement | arkings, Misc.: | \$ | 252,500.00 |
| Retaining Wall |  |  |  |  |  |  |  |
| 13530 |  | RW-3 Retaining Wall \& Excavation | 225 | CY | \$ 500.00 | \$ | 112,500.00 |
| Sub-Total for Retaining Wall: \$ |  |  |  |  |  |  | 112,500.00 |
| Maintenance of Traffic |  |  |  |  |  |  |  |
| N/A | N/A | MOT Lump Sum | 1 | LS | \$ 30,000.00 | S | 30,000.00 |
| Sub-Total for Maintenance of Traffic: \$ |  |  |  |  |  |  | 30,000.00 |
| Erosion and Sediment Control / Seeding |  |  |  |  |  |  |  |
| N/A | N/A | E\&S Lump Sum | 1 | LS | \$ 150,000.00 | \$ | 150,000.00 |
| Sub-Total for Erosion and Sediment Control / Seeding: |  |  |  |  |  | \$ | 150,000.00 |
| Traffic Signal |  |  |  |  |  |  |  |
| N/A | N/A | Traffic Signal Lump Sum | 1 | LS | \$ 350,000.00 | S | 350,000.00 |
| Sub-Total for Traffic Signal: |  |  |  |  |  | \$ | 350,000.00 |


| PROJECT BUDGET: OPTION CPROPOSED PUBLIC ROAD CONSTRUCTIONROUTE 250 (JEFFERSON HWY) TO SITE (APPROX. 5,000 LF)1/26/2018 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| CONSTRUCTION |  |  |  |  |  |
|  |  | LOW |  | HIGH |  |
| Pavement Demolition and Resurfacing, Saw Cut, and Earthwork | \$ | 5,216,440 | \$ | 5,216,440 |  |
| Storm Sewer and Hydraulics | \$ | 1,543,675 | \$ | 1,543,675 |  |
| Pavement and Stone | \$ | 1,131,900 | \$ | 1,131,900 |  |
| Curb, Pavement Markings, Misc. | \$ | 328,100 | \$ | 328,100 |  |
| Retaining Wall | \$ | 50,000 | \$ | 50,000 |  |
| Maintenance of Traffic | \$ | 75,000 | \$ | 75,000 |  |
| Erosion and Sediment Control / Seeding | \$ | 200,000 | \$ | 200,000 |  |
| Traffic Signal | \$ | 50,000 | \$ | 50,000 |  |
| Sub-Total A: | \$ | 8,595,115 | \$ | 8,595,115 |  |
| OTHER CONSTRUCTION BID COSTS |  |  |  |  |  |
| Mobilization for Sub-Total A (Calculated per VDOT formulas) | \$ | 459,756 | \$ | 459,756 |  |
| Materials Testing | \$ | 171,902 | \$ | 171,902 |  |
| Construction Staking / Surveying (2\%) | \$ | 171,902 | \$ | 171,902 |  |
| Right of Way and Utilities (35\% to 65\% Per VDOT formulas) | \$ | 3,008,290 | \$ | 5,586,825 |  |
| Sub-Total B: | \$ | 3,811,851 | \$ | 6,390,385 |  |
| Total Bid Items ( $\mathbf{A}+\mathbf{B}$ ): | \$ | 12,406,966 | \$ | 14,985,500 |  |
| Contingency for Construction and PE (25\% of Total Bid Items) | \$ | 3,101,741 | \$ | 3,746,375 |  |
| Sub-Total C (Total Bid Items + Contingency): | \$ | 15,508,707 CN | \$ | 18,731,875 | CN |
| TOTAL PROJECT BUDGET ( C) (ROUNDED) in 2018 dollars: | \$ | 15,508,700 | \$ | 18,731,900 |  |
| TOTAL PROJECT BUDGET ( C) (ROUNDED) in 2026 dollars: | \$ | 19,646,000 | \$ | 23,729,000 |  |

[^4]
## PROJECT BUDGET - DETAILED BREAKDOWN FOR OPTION C PROPOSED PUBLIC ROAD CONSTRUCTION ROUTE 250 (JEFFERSON HWY) TO SITE (APPROX. 5,000 LF)




[^0]:    Source: VDOT

[^1]:    * The total construction dollars above only includes items listed under the associated detailed breakdown.
    * Low vs High cost estimates reflect variance in ROW and Utilities only.

[^2]:    * The total construction dollars above only includes items listed under the associated detailed breakdown.
    * Low vs High cost estimates reflect variance in ROW and Utilities only.

[^3]:    *Inflation 3\% compounded annually for 8 years (3\%) to get from 2018 to 2026

    * The total construction dollars above only includes items listed under the associated detailed breakdown.

[^4]:    *Inflation 3\% compounded annually for 8 years (3\%) to get from 2018 to 2026

    * The total construction dollars above only includes items listed under the associated detailed breakdown.

